## \*27IHSSF1045\*



DocumentID

NONCD0002873

Site Name

**NELLO TEER QUARRY-DENFIELD** 

DocumentType Site Assessment Plan (SAP)

RptSegment

1

DocDate

10/28/1993

DocRcvd

2/20/2007

Вох

SF1045

AccessLevel

**PUBLIC** 

Division

**WASTE MANAGEMENT** 

Section

**SUPERFUND** 

Program

IHS (IHS)

DocCat

**FACILITY** 

## COMPREHENSIVE SITE ASSESSMENT

at

DURHAM QUARRY
Denfield Street
Durham County
Durham, North Carolina
DEM Facility # 0-012984
Groundwater Incident # 9357

for

Teer Company
Post Office Box 13983
Research Triangle Park,
North Carolina 27709-3983

by

GEONETICS CORPORATION

Payne Road Route 10, Box 2620 Lexington, N.C. 27292 Telephone (919) 764-9225 Suite 1 5120 S. Lakeland Drive Lakeland, Florida 33813 Telephone (813) 646-2644

Geonetics Corporation
October 28, 1993

# TABLE OF CONTENTS

	Page
EXECUTIVE SUMMARY	1
Purpose and Site History	1
Initial Abatement / Emergency Reponse Information	1
Nature and Extent of Release	4
SITE HISTORY & SOURCE CHARACTERIZATION	6
POTENTIAL RECEPTORS & MIGRATION PATHWAYS	10
REGIONAL GEOLOGY & TOPOGRAPHY	14
SITE ASSESSMENT	17
Soil Borings and Sampling	17
Groundwater Investigation	29
Groundwater Levels and Flow Directions	40
CONCLUSIONS AND RECOMMENDATIONS	45
REPORT CERTIFICATION	47
REFERENCES	48
APPENDIX Separate Bin	der

# LIST OF FIGURES

## **EXECUTIVE SUMMARY**

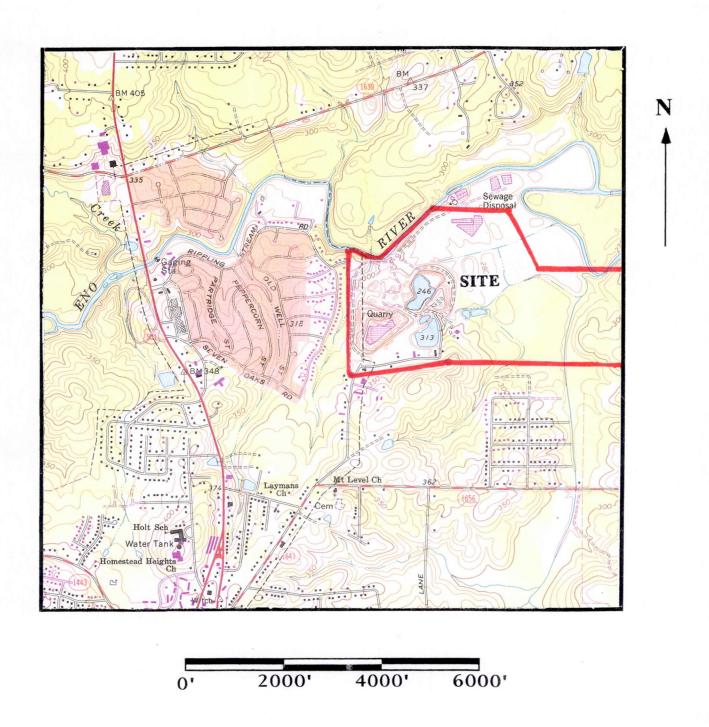
## Purpose and Site History

This CSA report presents the assessment and interpretations of a petroleum contamination study at a mining and processing site, the Durham Quarry, located at the end of Denfield Street (State Road 1641), north of Durham, in Durham County; Figures 1 and 2. An unknown amount of fuel leaked into the soil and groundwater, from the area of one or more of four gasoline tanks, two diesel tanks, and a used oil tank; and possibly the distribution lines and dispensers. The age of these tanks is not known. The fuel tanks were removed in December, 1988 when the service station was taken out of service.

The property is presently owned and operated by Teer Company. It has been in operation as a crushed stone quarry and processing plant since the 1940's, for use in highway and other construction materials. The North Carolina Department of Transportation originally owned and operated the site as a quarry and an asphalt batch plant. The property and these facilities were subsequently sold to Teer in 1951. The asphalt plant has since been relocated to another site. The N.C. Division of Environmental Management (DEM) has assigned Groundwater Incident # 9357, and the Site Facility # 0-012984.

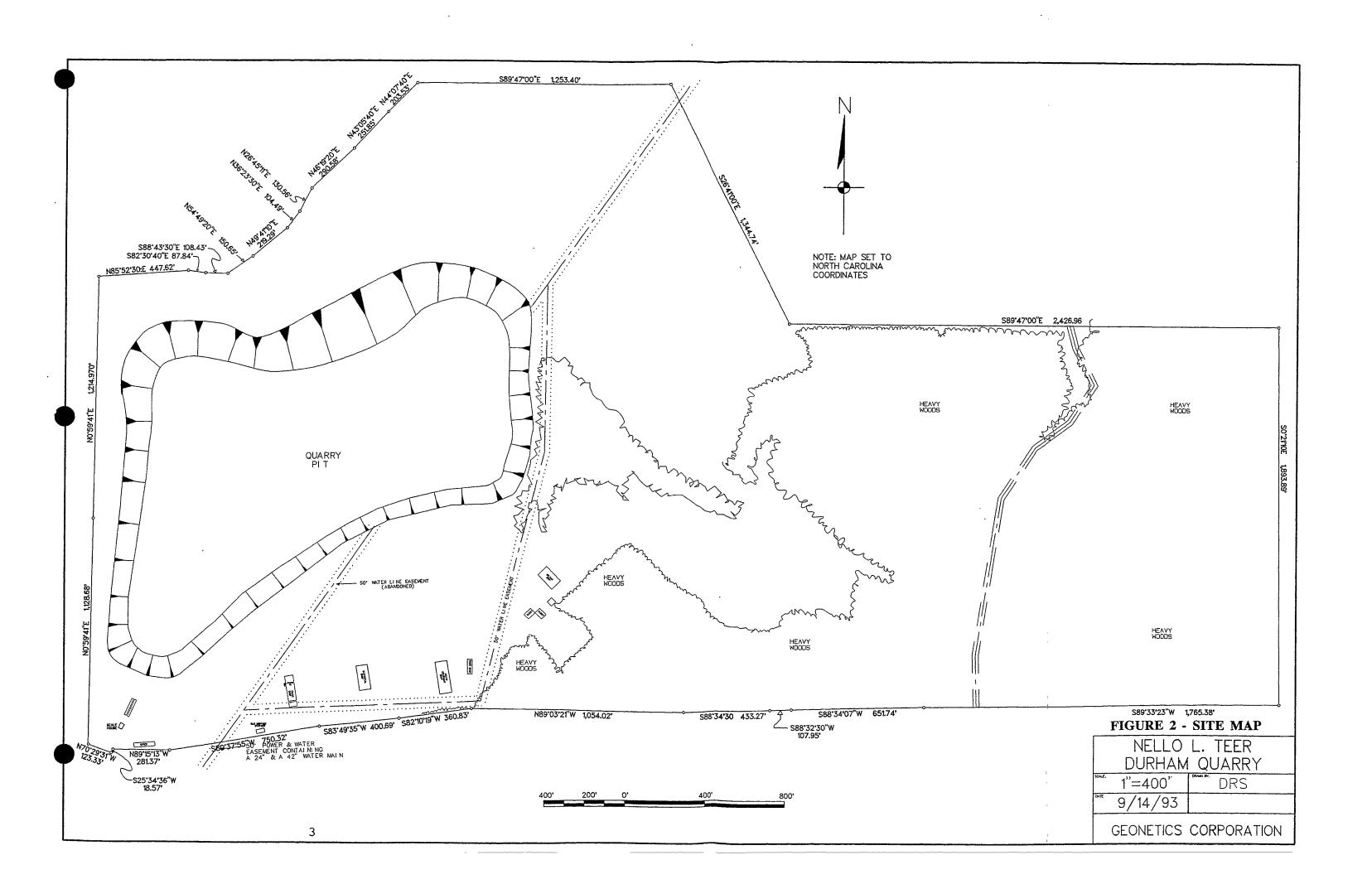
## Initial Abatement/Emergency Response Information

No free product was found during this investigation. There was no Initial Remedial Action on site, following the closure. Contamination was later suspected in one of the supply wells, and they were sampled for laboratory analyses. Notification of the contamination was submitted to DEM's Raleigh Regional Office on October 2, 1992.



Northwest Durham 7.5 min. Quad USGS, 1987 (Revised)

Figure 1 - Location Map



## Nature and Extent of Release

A well inventory has been completed which identified 15 supply wells within 1,500 feet of the site. Only one of Teer's on site supply wells is known to be affected by the contaminant migration from the release. The groundwater contamination appears to have remained on the site. There is no migration toward the Eno River, north of the property.

Laboratory results indicated that the greatest soil contamination is: TPH at 117 mg/kg (ppm) (EPA Method 5030); TPH at 2,800 mg/kg (EPA Method 3550); and TPH at 572 mg/kg (EPA Method 9071). These were located in the vicinity of the former service station and the truck shop at depths from 2 to 15 feet BLS. Current DEM Guidelines prescribe reportable amounts of soil contamination to be those above: 10 ppm (Method 5030); 40 ppm (Method 3550); and 250 ppm (Method 9071). EPA thresholds are: contaminated >2 ppm; and excessively contaminated >5 ppm, for TPH.

The highest groundwater contamination is: benzene, 672 ug/L (ppb); total VOA, 3,244 ug/L (ppb); in monitoring well 2 (MW-2) from a screened depth of 14 to 52 feet, BLS. One supply well (W-1) produced water containing benzene at 12 ug/L (ppb) and total VOA at 12 ug/L, from the most recent sampling (October 5, 1993). Current North Carolina Water Quality Standards (15A NCAC-2L .0202) has established a maximum allowable concentration of 0.001 mg/L (1 ug/L [ppb]) for benzene. The contaminant concentrations on this site have been decreasing steadily since first analyzed. Bottled drinking water is provided by the company for its personnel. That well water is only used for the toilets.

The shallow (perched) groundwater flows eastererly, due to a slight groundwater gradient on site and the dip of the underlying Triassic sedimentary rocks. The primary water table (unconfined) aquifer shows a south-southeasterly gradient in the vicinity of the former service station; except that supply well W-1 creates a large "cone" of depression upon the water table in this area and draws the surrounding contamination to itself. Estimated hydraulic conductivity ranges from 1 X 10<sup>-3</sup> cm/sec (2.8 ft/day) in the sand-silt horizons, to 1 X 10<sup>-5</sup> cm/sec (0.03 ft/day) in the clayey and silty sedimentary units.

An estimate of the volume of contaminated soil is 91,852 cubic yards (137,778 tons). Contaminated groundwater volume is estimated to be 31.12 million gallons (4.16 million cubic feet). It is proposed that the contaminated soils would be treated using bioremediation techniques on site; and that the contaminated groundwater could be remediated using in-situ sparging and/or pump and treat methods with activated charcoal. A Corrective Action Plan is being prepared for submittal by November 29, 1993.

This site is eligible for the State Trust Fund reimbursement program. Following submittal of this CSA, an application for reimbursement will be prepared for payment above the "deductable" requirement.

## SITE HISTORY & SOURCE CHARACTERIZATION

The subject site is presently owned and operated by Teer Company. Previously, the site had been owned and operated by the North Carolina Department of Transportation as a crushed stone quarry, processing plant, and asphalt batch plant, since the 1940's. In 1951, Teer purchased the property. The asphalt plant was operated until 1990, when it was shut-down and relocated.

In 1986, Teer Company registered as owner of the six active underground storage tanks (UST's) on the property. The four gasoline tanks and two diesel tanks at the former service station were emptied and removed in December, 1988. They were: one 3,000 gallon gasoline, two 4,000 gallon gasoline, one 10,000 gallon gasoline, and two 10,000 gallon diesel tanks. There has been no initial remedial action at the subject site to date. No free product was encountered during this investigation.

An unusual taste and odor in the well water at the Quarry Office prompted sampling of the supply wells. Laboratory analysis found constituents of gasoline present in those samples, which was then reported to DEM in October, 1992. On January 25, 1993, a Notice of Violation was issued by the Raleigh Regional Office of DEM to Teer Company with respect to Title 15A NCAC-2L .0202, and G.S. 143-215.1. Teer contracted the services of Geonetics Corporation of Lexington, North Carolina and Lakeland, Florida to conduct the site investigation. The site activities began in March, 1993, and involved soil borings, sampling and field screening of soils around the gasoline UST's, in the southern area of the quarry property. New monitoring wells were planned, drilled and constructed so as to sample the groundwater for assessment and compliance purposes.

Immediately after petroleum hydrocarbons were first detected in the potable well (W-1), located between the Quarry Office and the truck shop, the company began supplying bottled drinking water for personnel. This well water is now only utilized for flushing toilets.

Geonetics performed a Site Sensitivity Evaluation (SSE) early in the assessment phase in accordance with DEM Guidelines. The purpose of an SSE is to evaluate the susceptibility of groundwater to contamination from petroleum-contaminated soils. It is used as a method of determining the degree of urgency in response to a spill or leak. The following DEM forms constitute the revised SSE.

-7-

## Site Sensitivity Evaluation (SSE)

Site Characteristics Evaluation (Step 1)

Characteristic	Condition	Rating	
Grain Size*	Gravel Sand Silt Clay	150 100 50 0	50
Are relict structures, sedimentary structures, and/or textures present	Present and intersecting the water table.  Present but not	10	
in the zone of contamination and underlying "soils"?	intersecting the water table.  None present.	5 O	10
Distance from location of deepest contaminated soil** to water table.	0 - 5 feet (C, D & E sites only) 5 - 10 feet >10 - 40 feet >40 feet	20 20 10 0	20
Is the top of bedrock or transmissive indurated sediments located above the water table?	Yes No	20 0	20
Artificial conduits present within the zone of contamination.	Present and intersecting the water table. Present but not intersecting the water table.	10 5	10
	None present.	0	10

Total Site Characteristics Score: 110

<sup>\*</sup> Predominant grain size based on Unified Soil Classification System or U.S. Dept. of Agriculture's Soil Classification Method.

<sup>\*\*(&</sup>gt;10ppm TPFH by Method 5030; >40 ppm TPFH by Method 3550;

<sup>&</sup>gt;250 ppm O&G by Method 9071)

# Site Sensitivity Evaluation (SSE)

Initial Clea (Ste	~				leanup Lev Step 3)	/el
(ote)	Low	Boiling	Poi	od 5030 for nt Hydrocarbons tion Fuels, and Ga	antaniananananananananananananananananan	Final Cleanup
Total Site Characteristics Score	Initial Cleanup Level TPFH (ppm)			Category A & B (Multiply initial cleanup level by 1)		Level $0 = 40$ ppm
>150 121-150 91-120	≤10 20 40	Selec Site		Category C & D (Multiply initial cleanup level by 2)	2 X	= ppm
61-90 31-60 0-30	60 80 100	Catego		Category E (Multiply initial cleanup level by 3)	3 X	= ppm

		Boiling, Diesel,	Point Varso	3550 for Hydrocarbons ol, Mineral Spirits,	Naph	ntha Final
Total Site Characteristics Score	Initial Cleanup Level TPFH (ppm)			Category A & B (Multiply initial cleanup level by 1)	1 X <u>1</u>	Level $\underline{60} = \underline{160}$ ppm
>150 121-150 91-120	≤40 80 160	Select Site		Category C & D (Multiply initial cleanup level by 2)	2 X _	= ppr
61-90 31-60 0-30	240 320 400	Categor	y*	Category E (Multiply initial cleanup level by 3)	3 X _	= ppn

	such as Fuel Oil	Fuels -	Oil #6,	od 9071 for & Grease (O&G) Motor Oil, Hydrau	ulic Fluid	Final Cleanup
Total Site Characteristics Score	Initial Cleanup Level TPFH (ppm)			Category A & B (Multiply initial cleanup level by 1)	1 X <u>550</u>	Level = <u>550</u> ppm
>150 121-150 91-120	≤250 400 550	Selec Site	t	Category C & D (Multiply initial cleanup level by 2)	2 X	= ppm
61-90 31-60 0-30	700 850 1000	Categor	у*	Category E (Multiply initial cleanup level by 3)	3 X	= ppm

<sup>\*</sup> See Site Category Descriptions, Table 3 NCDEM Guidelines 3/10/93

#### POTENTIAL RECEPTORS & MIGRATION PATHWAYS

A walkover survey, interviews, and a search of County Health Department well records were used in conjunction with the USGS topographic map of the Northwest Durham 7.5 minute Quadrangle to locate water supplies within 1,500 feet of the subject site (USGS, Revised, 1987). Fifteen potential receptor wells, including the five supply wells on site, were found within a 1,500-foot radius of the former service station. A list of the well owners is provided in Table 1, and is keyed to the Well Inventory Map, Figure 3.

Table 1. - POTENTIAL RECEPTOR WELLS

OWNER/RESIDENT

# Aron 4807 Denfield Street Church of God Denfield Street Albert Lee Deer 4911 Denfield Street Wright 4907 Denfield Street W.T. Proctor, Inc. 4918 Denfield Street Walters 4921 Denfield Street

Lee's Welding 1002 Communication Drive

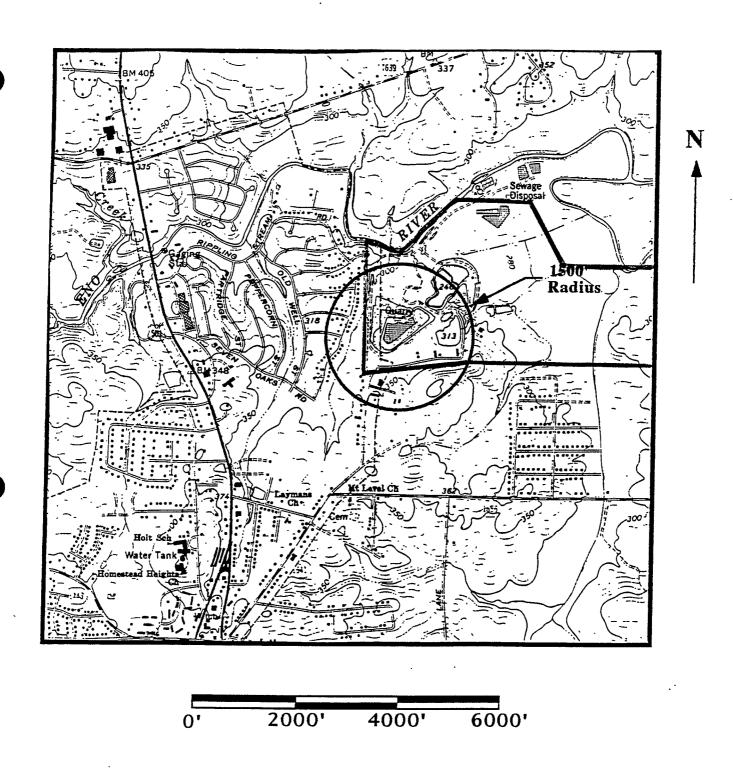
Joann Harris- Mobile Communications 1003 Communication Drive

D.W. Ward Construction Co. Denfield Street

Mayo Farms Trucking Co. 4934 Denfield Street

Teer Company, 5 supply wells Durham Quarry

**ADDRESS** 



Northwest Durham 7.5 min. Quad USGS, 1987 (Revised)

Figure 3 - Well Inventory Map

The only identified surface water for a public water supply is the Eno River, located approximately one-half mile north of the former service station. It flows in an easterly direction into the Neuse River. The large quarry pit separates the contaminated area from the Eno River. An out-of-service water supply inlet, owned by the City of Durham, is downstream from the sewage treatment plant, and has been disconnected for approximately three years (personal communication, Durham Water Resources Division, October 11, 1993).

Municipal water is available to other area residences and businesses from the City of Durham Public Works Department. The local water main turns east at Hebron Road (SR 1656) off of Denfield Street, approximately 3,000 feet south of the Quarry entrance. Two water supply pipelines, 24-inch and 42-inch diameter, are deeply buried in a trench which traverses the Teer property along a 50-foot easement granted to the City of Durham (see Figure 2). These mains supply the areas north of the Eno River. A sewer main also traverses the eastern side of the Teer Property along a 30-foot easement, to bring wastewater to the City's treatment plant adjacent to the Eno.

The 327 acre subject site is bordered on the north by the City of Durham wastewater treatment plant property. Table 2 is a list of adjacent property owners, obtained from the Durham County Courthouse. There are no basements constructed on the site. The Quarry Office and shops are on septic tank systems which are located near the buildings.

Table 2. - ADJACENT PROPERTY OWNERS

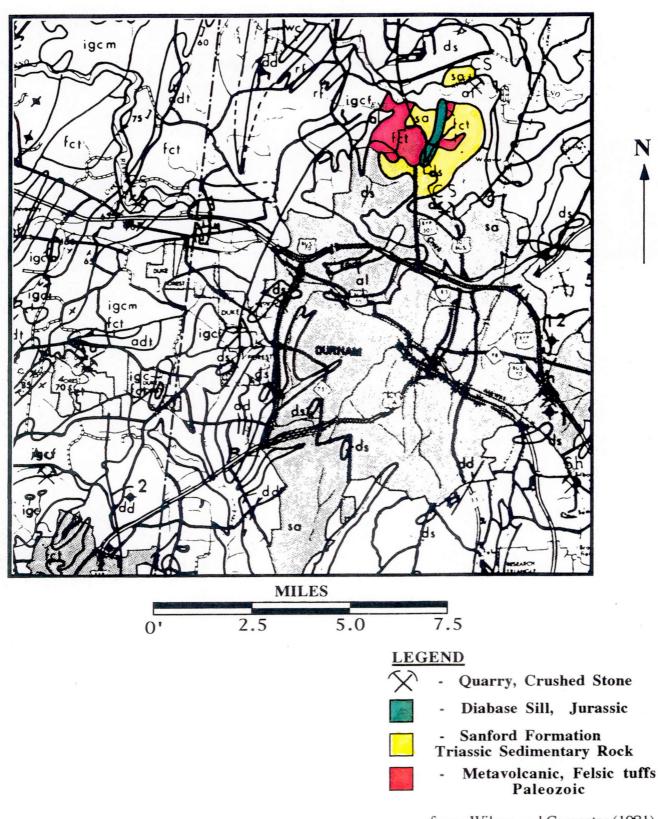
<u>MAP</u>	BLOCK	<u>LOT</u>	<u>NAME</u>	MAP	<u>BLOCK</u>	LOT	NAME
801	1	4 10 8B 9 17C 17D 1E 3A	Penny, Jack B. City of Durham City of Durham McKarland, W.C. City of Durham	785 788	2 3	1 1,2,3 4 5 6 7 8 9	Royster, Gail Nelson, C.W. Howell, R. Holmes, Leroy Riley, John L. Harpe, James Stanfield, W.J Cheek, Wm.
787	1	1 2 5 5A	Central Engg. & Con Central Engg. & Con City of Durham City of Durham			10A 12 13,14 15 16,17	Noell, Eula A. White, Lester Overby, C.H. Hart, Bertha Hart, Wm. D. Tate, Grady
747	1	10	Pendergrass, Bobby	F.		18,19,20	Bragg, J.H.
792	4 5	15 16 1 2	Cook, David L. Flack, C.V., Jr. Williams, Edgar W. Lett, Clyde			21 22 23 24 25	Wicker, A.D. Hall, Roose. Hall Estate Nunn, Edgar L Solomen, Vert
		1 2 3 4 5 6 7 8 9 10 11 12 13 14 15 16 17 18	Alston, Milton Mallard, Harvey M. O'Neal, Elton M. Richardson, Nathania Lemonjello, Gregory Roberson, Robert L. Gilmore, Exter G., Jr Lassiter, Barbara J. Johnson, Edwin V. Neal, Andrew G. Faulken, Tracy C. Davis, Harold R. Smith, Sherwood E. Bennett, Thomas S. Miles, Lonnie J. Bell, Tacho Peace, Larry D.		6 7	1 2 3 4 5 6 7 8 9 10 11 12 13 14 15	Durham City Gilbert, K.R. Egerton, G.G. Irvin, John L. I.R.T. Prop. Peace, Frankie Davis, Velma Davis, F.W. Dennis, A. King, B.B. Cook, Jimmie Smith, Wilma Young, C.J. Mitchell, G.A. Brown, Henry Long, Clyde
785	1	1 22 23 24 25	Swann, Louis Johnson, Danny B Bankston, Mildred G Wright, MacArthur Johnson, Fred, Jr.	780	1	1 2 2 4 5	Jones, P.W. Joven Group 7 Central Engg. Central Engg.

#### REGIONAL GEOLOGY & TOPOGRAPHY

The project site is situated within the Durham Basin, the northernmost portion of the Deep River Basin, within the Piedmont Physiographic Province of central North Carolina (Hoffman and Gallagher, 1989). This half-graben structural basin lies between the Western Slate Belt and the Raleigh Belt, of the Piedmont.

Surficial rocks in the area of the site have been mapped as the late Triassic-age siltstones, sandstones, mudstones, shales and fanglomerates, belonging to the Sanford Formation, Chatham Group (Hoffman and Gallagher, 1989) Figure 4. These sedimentary rocks are typically maroon, tan, red and reddish-brown. Abundant outcrops are observed in the immediate vicinity of the site. The regional strike of the Triassic strata is N40° -50°E; with a southeasterly dip of 7 to 10 degrees. Thicknesses of the Triassic rocks range from zero to more than 280 feet toward the eastern side of the site.

Disconformingly below the Triassic sediments, are two very different rock units, both of which have been utilized, at different times, as quarry stone. The oldest and most durable rock consists of a suite of metavolcanics (felsic tuffs) of Paleozoic age. These are a very dark blue-gray, fine grained rock, which weathers with an iron-brown staining. The younger rock is an igneous diabase of early Jurassic age which was intruded through and into the metavolcanic and sedimentary rocks. The diabase has been emplaced as dikes, cutting across strata, or as sheets (sills) between sedimentary strata. This rock is dark gray to black, medium grained, and is classified as a diabase because its crystal grain sizes are between those of a gabbro and a basalt (Hoffman and Gallagher, 1989).



from: Wilson and Carpenter (1981)

Figure 4 - Regional Geologic Map

The soil developed from the Triassic sedimentary rocks is very thin to none. Most of the natural surficial material consists of reddish-tan, micaceous, silty soil, which is a deeply weathered sedimentary rock. Man-placed crushed stone and asphalt paving covers most of the quarry area.

The site is located on a wide topographic terrace (elevations 300 to 330 feet), between a large hill reaching an elevation of 397 feet, National Geodetic Vertical Datum (NGVD) south of the property, and the valley of the Eno River, flowing at an elevation of 262 feet. Quarrying operations over the past 50 years have modified the natural surface drainage. The regional drainage is eastward toward two small creeks and thence into the Eno River.

Usually, the groundwater gradient mirrors the surface topography and runoff. This is not true at this site because of the quarrying operations exceeding a depth of 180 feet BLS, the structural features (joints and faults), the Jurassic diabase intrusion, and the influence of supply wells on site. The average depth to the water table near the center of the contaminated area is 22.7 feet. However, the drilling of monitoring wells showed that the depths to groundwater range from 2 feet to more than 52 feet.

The shape of the groundwater contamination plume is elliptical, with its long axis parallel to the southern property line. It will be discussed later that the influence of supply well W-1 has pulled the contaminated groundwater downward to 260 feet, BLS, and northward toward itself, thus keeping the contamination from migrating offsite.

#### SITE ASSESSMENT

## Soil Borings and Sampling

The soil/sedimentary rock assessment required 112 borings, 14 hand auger borings and 11 test pits in order to completely define the area and depth of contamination. Auger flights and drilling tools were decontaminated prior to beginning each new bore hole in accordance with Geonetics' Quality Assurance and DEM's sampling protocol. Representative samples were retrieved from stainless-steel split-spoons or stainless-steel hand augers and placed in cleaned sample jars for both field and laboratory analysis. Field soil assessment screening was accomplished by use of Photo-Vac MicroTip PID and Heath Porta-FID III Organic Vapor Analyzers (OVA).

Fifty-one soil samples were obtained from borings and test pits for the requisite laboratory analyses, performed by Southern Testing and Research Laboratories of Wilson, N.C. Laboratory analytical methods included EPA 5030 and EPA 3550; and other areas sampled included EPA Method 9071 and EPA Method 1311-TCLP for the eight RCRA metals. The laboratory results are summarized in Table 3. These were utilized to confirm the broader results of the field -screening OVA methodology. The laboratory results and chains of custody are contained in the Appendix. The OVA field reporting forms and Boring Logs are in the Appendix of this report.

The soil OVA measurements are summarized in Table 4, and are depicted in Figure 5. Generally, 10 ppm by OVA method, and 5 ppm TPH by lab analysis indicates soil contamination from gasoline. Figure 5 also illustrates the horizontal extent of soil contaminated by petroleum product leaks.

Table 3 - SUMMARY OF SOIL LABORATORY ANALYSES FOR THE TRUCK SHOP AREA Teer Company, Durham Quarry, N.C.; Geonetics Project # 10293

PARAMETER (EPA METHO (UNITS) :	(D) >		8021 (601) (ug/kg)	8021 (602) (ug/kg)	9071 (mg/L)	TCLP 1311 (mg/L)							
BORING #	DEPTH (ft.)	SAMPLE DATE		TOTAL VOA	<b>TPH</b>	As	Ba	Cd	Cr	Pb	Hg	Se	Ag
B-85	5	5/25/93	ND	53.6	572	ND							
B-86	10	5/25/93	ND	82.7	113	ND	1.59	ND	ND	ND	ND	ND	ND
B-86	15	5/25/93	ND	76.6	172	ND	3.04	ND	ND	ND	ND	ND	ND
B-86	20	5/25/93	ND	92.7	54.5	ND	1.22	ND	ND	ND	ND	ND	ND
Near B-86	2.5	5/25/93	ND	73.8	122	ND	1.76	ND	ND	ND	ND	ND	ND
B-87	5	5/25/93	ND	71.8	183	ND							
B-87	7	5/25/93	ND	116.6	128	ND							
TP-1	2.5	5/25/93	ND	97.3	ND	ND	ND	ND	NID	ND	ND	ND	ND

ND=NOT DETECTED

ug/kg = micrograms per kilograms

mg/L = milligrams per Liter

Southern Testing & Research Laboratories, Inc., Wilson, N.C.

GEONETICS CORPORATION

Table 3 - SUMMARY OF SOIL LABORATORY ANAYLSES

Teer Company, Durham Quarry, N.C., Geonetics Project # 10293

PARAMETERS (EPA METHOD (UNITS) > BORING #	)) >	SAMPLE DATE	8021 (5030) (ppm) TPH	8015 (3550) (ppm) Kerosene Group	8015 (3550) (ppm) Gasoline <u>Group</u>
BORING #	DEFIN (it.)	JATE 1			
TP-2	COMP	6/15/93	7.579	52	<2
TP-3	COMP	6/15/93	0.577	<2	<2
TP-4	COMP	6/15/93	0.526	<2	<2
TP-5	COMP	6/15/93	0.849	<2	<2
TP-6	COMP	6/17/93	0.605	<2	<2
TP-7	COMP	6/17/93	0.058	<2	<2
TP-8	COMP	6/17/93	0.503	<2	<2
TP-9	COMP	6/17/93	0.742	2000	< 20
TP-10	COMP	6/17/93	1.603	<2	<2
TP-11	COMP	6/17/93	1.278	<2	<2
D-1	COMP	6/15/93	24.21	820	< 5
D-2	COMP	6/15/93	1.378	22	<2
D-3	COMP	6/15/93	0.401	340	< 5
D-4	COMP	6/15/93	116.8	2800	< 25
D-5	COMP	6/15/93	0.412	<2	<2
D-6	COMP	6/15/93	< 0.275	<2	<2
D-7	COMP	6/15/93	1.055	<2	<2
D-8	COMP	6/15/93	0.571	<2	<2

Table 3 - SUMMARY OF SOIL LABORATORY ANAYLSES

Teer Company, Durham Quarry, N.C., Geonetics Project # 10293

PARAMETERS (EPA METHO (UNITS) >	<b>D</b> ) >		8021 (5030) (ppm)	8015 (3550) (ppm)	8015 (3550) (ppm)	(9071) (mg/kg)
BORING #	DEPTH (ft.)	SAMPLE DATE	TPH	Kerosene Group	Gasoline Group	TPH
D-9	COMP	6/15/93	< 0.275	<2	<2	N/A
D-10	COMP	6/15/93	0.455	<2	<2	N/A
D-11	COMP	6/15/93	10.717	<2	<2	N/A
D-12	COMP	6/16/93	0.468	<2	<2	N/A
D-13	COMP	6/16/93	0.624	<2	<2	N/A
D-14	COMP	6/16/93	0.434	<2	<2	N/A
SB-1	2 to 4	7/19/93	0.903	<2	<2	N/A
SB-1	5 to 7	7/19/93	0.793	<2	<2	N/A
SB-1	10 to 12	7/19/93	0.479	<2	<2	N/A
SB-1	15 to 17	7/19/93	0.452	<2	<2	N/A
SB-1	20 to 22	7/19/93	0.479	< 2	<2	N/A
SB-2	2 to 4	7/20/93	0.439	<2	<2	N/A
SB-2	5 to 7	7/20/93	0.419	<2	<2	N/A.
SB-2	10 to 12	7/20/93	0.585	<2	<2	N/A
SB-2	15 to 17	7/20/93	0.446	<2	<2	N/A
SB-2	20 to 22	7/20/93	63	31	<2	N/A
SB-3	2 to 4	8/4/93	< 0.275	<2	<2	225
SB-3	5 to 7	8/4/93	< 0.275	<2	<2	105
SB-3	10 to 12	8/4/93	<0.275	<2	<2	132

Table 3 - SUMMARY OF SOIL LABORATORY ANAYLSES

Teer Company, Durham Quarry, N.C., Geonetics Project # 10293

PARAMETERS	S >		8021	8015	8015	
(EPA METHO)	D) >		(5030)	(3550)	(3550)	(9071)
(UNITS) >			(ppm)	(ppm)	(ppm)	(mg/kg)
BORBIG #	70.75170.05557	SAMPLE	TPH	Kerosene	Gasoline	TPH
BORING #	DEPTH (ft.)	DATE	······································	Group	Group	
SB-3	15 to 17	8/4/93	0.369	<2	<2	90.3
SB-3	20 to 22	8/4/93	0.304	<2	<2	167
SB-4	2 to 4	8/4/93	< 0.275	<2	<2	N/A
SB-4	5 to 7	8/4/93	0.279	<2	<2	N/A
SB-5	2 to 4	8/4/93	< 0.275	<2	<2	N/A
SB-5	5 to 7	8/4/93	< 0.275	<2	<2	N/A
SB-5	10 to 12	8/4/93	< 0.275	<2	< 2	N/A
SB-5	15 to 17	8/4/93	0.323	<2	<2	N/A
MW-9 LOC. *	2 to 4	7/2/93	2.45	<2	<2	N/A
MW-9 LOC. *	5 to 7	7/2/93	9.581	200	<2	N/A
MW-9 LOC. *	10 to 12	7/2/93	19.78	775	<2	N/A
MW-9 LOC. *	15 to 17	7/2/93	0.888	<2	<2	N/A
MW-9 LOC. *	20 to 22	7/2/93	0.54	<2	<2	N/A
MW-9 LOC. *	25 to 27	7/2/93	1.407	<2	<2	N/A

ppm = parts per million

TP = Test Pit

D = Ditch Area Located on the South end of Property

SB = Soil Boring

\* = Soil Removed During MW-9 Installation

mg/kg = milligrams per kilograms

Table 4 - SUMMARY OF SOIL OVA DATA (parts per million) er Company, Durham N.C.; Geonetics Project # 10293

DEPTH (ft.) >		5	10	15	20	25	30	35	40
BORING #	DATE	<u> </u>			· · · · · · · · · · · · · · · · · · ·				<del></del>
B-1	4/23/93	0	0						
B-2	4/23/93	0	1	12					
B-3	4/23/93	0							
B-4	4/23/93	1000	18	26	26				
B-5	4/23/93	0							
B-6	4/23/93	9.5	15	1	0	0	6.5		
B-7	4/24/93	28	13	1					
B-8	4/24/93	7							
B-9	4/24/93	210	560	117	34				
B-10	4/24/93	5.5							
B-11	4/24/93	665	152	61	29	72	11	1	0
B-12	4/24/93	110	290	2	2	12	7	14	
B-13	4/24/93	570	537	16					
B-14	4/24/93	100	58	3	2				
B-15	4/25/93	125	2	5	12				
B-16	4/25/93	7.5							
B-17	4/25/93	60	23	26	3	4.5			
B-18	4/25/93	480							
B-19	4/25/93	190	450						
B-20	4/25/93	1000	240	120	15.5	28.5	16	38	
B-21	4/25/93	12	540						
B-22	4/25/93	590	96	41	17				
B-23	4/25/93	170	30	86					
B-24	4/25/93	74	45	270					
B-25	4/25/93	17	63	210					
B-26	5/1/93	4	32.5						
B-27	5/1/93	65	61						
B-28	5/1/93	18.5	77						
B-29	5/1/93	5	11						
B-30	5/1/93	15.5	21.5						
B-31	5/1/93	4.5	8.5						
B-32	5/1/93	25	4	2.5					
B-33	5/1/93	1	4						
B-34	5/1/93	25.5	3						
B-35	5/1/93	41	65						
B-36	5/1/93	11	70	80					
B-37	5/1/93	17	31	18.5	12				
B-38	5/1/93	4.5	41.5	33	60.5				
B-39	5/1/93	5							
B-40	5/1/93	1	1	140					

Table 4 - SUMMARY OF SOIL OVA DATA (parts per million) er Company, Durham N.C.; Geonetics Project # 10293

DEPTH (ft.) >		5	10	15	20	25	30	35	40
BORING #	DATE								
B-41	5/2/93	4.5	2	68					
B-42	5/2/93	0	1	440					
B-43	5/2/93	21							
B-44	5/2/93	5.5	6						
B-45	5/2/93	0	6.5						
B-46	5/4/93	2	2	18	8	5	11	8	
B-47	5/5/93	142/2*	26/4*						
B-48	5/5/93	1	0	2.5	360				
B-49	5/5/93	39	350	650					
B-50	<i>5/5/</i> 93	1	2	2					
B-51	5/6/93	0	1	0	0				
B-52	5/6/93	0	0						
B-53	5/6/93	0	0						
B-54	5/7/93	1.5	1						
B-55	5/10/93	0							
B-56	5/15/93	6.5	188	2191					
B-57	5/15/93	1.3	2.2	0.4					
B-58	5/15/93	15.1	1.9	8.4					
B-59	5/15/93	12.1							
B-60	5/15/93	9.4	41.6	10.6	32				
B-61	5/15/93	5.6	14.4	58.2					
B-62	5/16/93	3.1							
B-63	5/16/93	2.6	4						
B-64	5/16/93	1629	64.1						
B-65	5/16/93	15.8	4.7						
B-66	5/16/93	1.5							
B-67	5/16/93	16.9							
B-68	5/16/93	3.7	3.8						
B-69	5/16/93	3.9	3.1	4.1	3.2				
B-70	5/16/93	0	1.1	2.9	3.5	5	1.7		
B-71	<i>5</i> /17/93	0.3	1	1.3	1.1				
B-72	5/17/93	12							
B-73	5/17/93	6.8	17.6/6*						
B-74	5/17/93	29.3	4.7	3.2	2.1	2500+			
B-75	5/22/93	3.8	2.2	4.4	4.9	20			
B-77	5/21/93	10.5	5.5	2.1	10.2				
B-78	5/21/93	5.2	72.5	13.1	10.2	9.8			
B-81	5/22/93	4.4	4.7	5.3	2.5				
B-82	5/22/93	5.6	3.3	12.1	4	3.1	4		
B-83	5/22/93	5.2	2.8						

Table 4 - SUMMARY OF SOIL OVA DATA (parts per million) er Company, Durham N.C.; Geonetics Project # 10293

<b>DEPTH</b> (ft.) >		5	10	15	20	25	30	35	40
BORING #	DATE	4							
B-84	5/22/93	5.9	4.8						
B-85	5/25/93	21.2							
B-86	5/25/93	8.6	5.5	4.1					
B-87	5/25/93	4.7							
B-88	5/26/93	6.7	1.9	2.9	2.4	2.8	2.1		
B-89	5/26/93	2.8	2.3	4.6	5.2				
B-91	<i>5</i> /26/93	2.8	2.7	2.9	4.5	3			
B-92	6/1/93	4.8	3.4	2.5	8.2	3			
B-93	6/2/93	3.9	4.7	9.5					
B-94	6/2/93	1							
B-95	6/1/93	2.2	1.1						
B-96	6/1/93	2.1	2	2.8	2.5	2.8			
B-97	6/1/93	5.7	2.5						
B-98	6/1/93	4.1							
B-99	6/2/93	2.1	1.9	1.5					
B-100	6/2/93	1.2	3	6.2	26.3	5.5	2.9		
B-101	6/3/93	2.4	2.3	3.4	5.4	4.9	4.2		
B-106	6/3/93	2.6	2.3	2.6	10.1				
B-107	6/4/93	8	2						
B-108	6/3/93	2.6	2.7						
B-109	6/4/93	7.4	20.1	22.4					
B-110	6/3/93	5.7	3.8	6.4	5.3	13.2	7.8		
B-111	6/3/93	2	9.9						
B-112	6/3/93	12.3	9.8						
B-113	6/4/93	2.9	1.6	2	11.5				
B-114	6/7/93	37.9	8.1	4.1	4	2			
B-115	6/7/93	0.5	2.7	1.2	0.7	0.6	0.7		
B-116	6/4/93	1.8	1.5	2	2.5				
B-117	6/4/93	1.9	4.1						

INSTRUMENT USED: PHOTOVAC MICROTIP MP1000 P.I.D. METHOD USED: HEADSPACE

<sup>\*=</sup>PPM/DEPTH

Table 4 - SUMMARY OF SOIL OVA DATA (parts per million) er Company, Durham N.C.; Geonetics Project # 10293

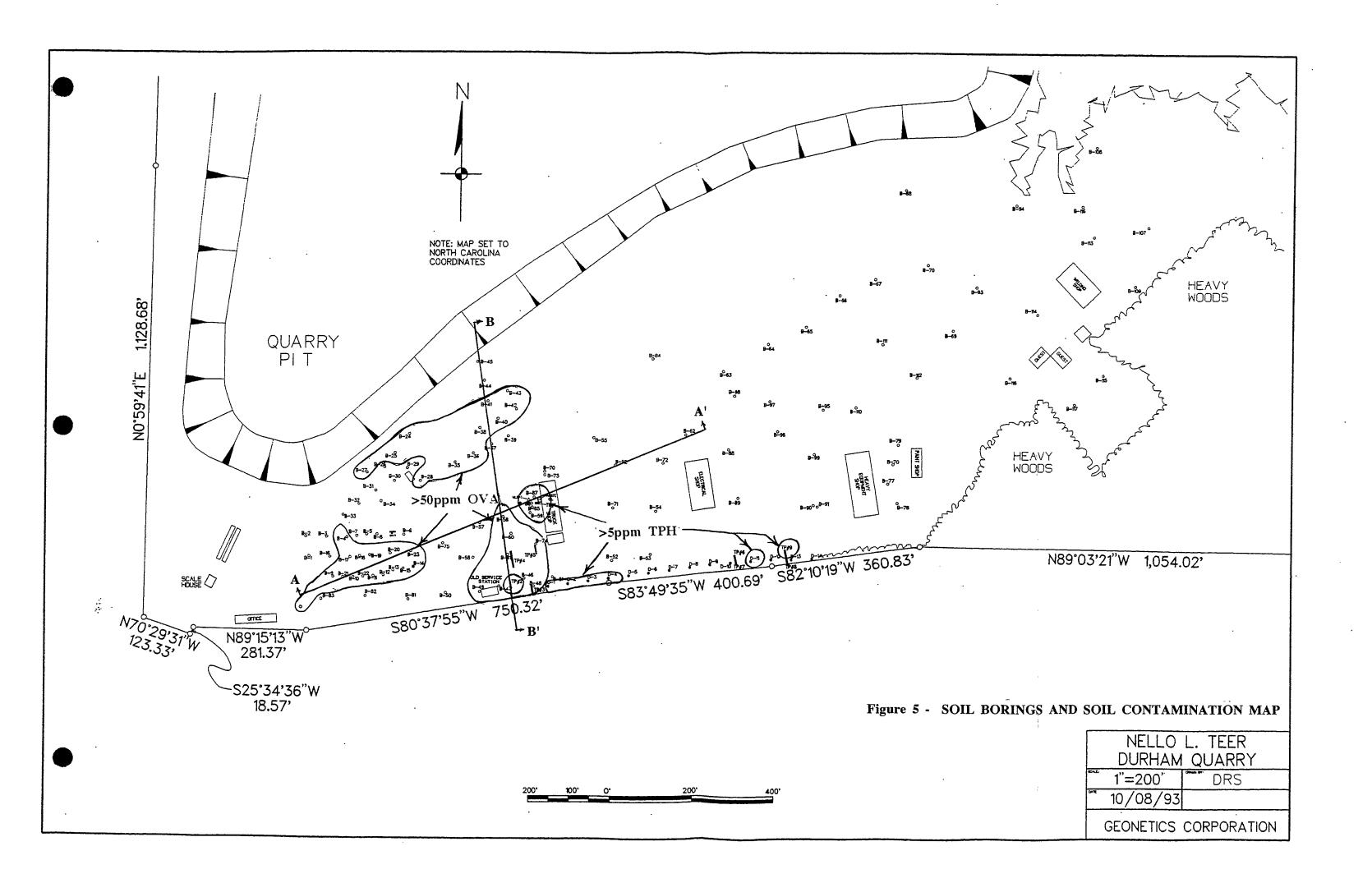
<b>DEPTH</b>	(ft.) >	1	2	3	4	5	6	7	8	9	10	11	12	13
BORING #	DATE													
TP-1	5/25/93		14.8											
TP-2	6/14/93	18.7	0	0	0	0.2								
TP-3	6/14/93	0	2.3	0										
TP-4	6/14/93					0	0	0	0.7	0	0	0	0	0
TP-5	6/14/93							0	0	0	0			
TP-6	6/17/93							0			0	0		
TP-7	6/17/93							0	0	0				
TP-8	6/17/93					0	0	0	0					
TP-9	6/17/93									0	0	0		
TP-10	6/17/93						0	0						
TP-11	6/17/93								0	0	0			
D-1	6/14/93	68.8	19.6	9.1										
D-2	6/14/93	16.1	18.1	15.2										
D-3	6/14/93	10.2	4.8											
D-4	6/14/93	129												
D-5	6/14/93	3.4	2											
D-6	6/14/93	0	0											
D-7	6/15/93	0	0											
D-8	6/15/93	0	0											
D-9	6/15/93	0	0											
D-10	6/15/93	0	3.1	0										
D-11	6/15/93	0	0	0										
D-12	6/16/93	0	0	0	0									
D-13	6/16/93	0	0											
D-14	6/16/93	0	0.2	0	0									

TP = TEST PIT

D=DITCH(ALONG SOUTH SIDE OF PROPERTY)

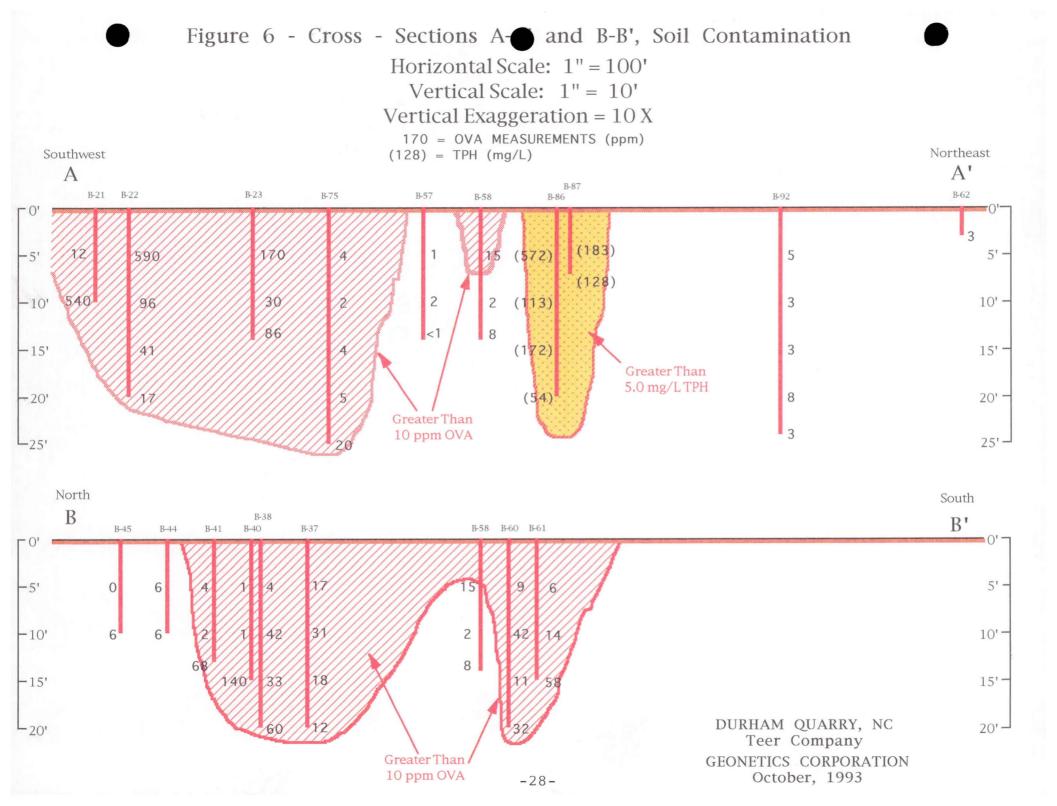
INSTRUMENT USED: PHOTOVAC MICROTIP MP1000 P.I.D.

METHOD USED: HEADSPACE



Cross-sections A-A' and B-B', Figure 6, illustrate the vertical distribution of contaminated soil (sedimentary rocks). The deepest soil sample in each bore hole was obtained from approximately one foot above the water table, or above the point of auger refusal. The configuration of soil contamination supports the interpretation that fuels migrated downward toward the water table, initially. Then, as the groundwater migrated, the contaminants were transported along with it. During periods of seasonal water table fluctuations, the contaminants were brought up into previously clean soils, and deposited there.

The areas which will require soil remediation are shown on Figure 5 and include: the former service station; the present septic tank drain field; the former asphalt plant area; an area in front of the truck shop; and portions of that drainage ditch along and near the southern property line. The EPA threshold of 5 mg/kg for TPH and the OVA threshold of 50 ppm were used to calculate the volume of excessively contaminated soil. The sum of all areas is 124,000 ft<sup>2</sup> times an average contaminated thickness of 20 ft., gives a volume of 2.48 million ft<sup>3</sup>, which is approximately 91,852 cubic yards. A density estimate of 1.5 tons per cubic yard was utilized to calculate a weight of 137,778 tons.



## **Groundwater Investigation**

The field-screening OVA method guided the placement of the new monitoring wells. Five supply wells and eight monitoring wells already existed on site, and 17 new groundwater assessment monitoring wells were installed in order to define the lateral and vertical extent of this contaminant plume, and the degree of contamination. Decontamination of the drill rig and tools was performed in accord with Geonetics' QA protocol before beginning each new hole, so as not to cause any cross-contamination between wells. The Well Construction Diagrams are contained in the Appendix. Table 5, following, summarizes the construction details of each production and monitoring well.

Monitoring well drilling was performed by a down-hole hammer rig. The cuttings were stockpiled and covered for later treatment. After each monitoring well was completed with sand pack, bentonite seal and a grout seal, it was developed until the groundwater was clear. Prior to sampling, purging of the well was performed to ensure that the water sample was representative of that screened portion of the aquifer. Groundwater sampling was accomplished by use of sterile disposable bailers, to prevent the introduction of any other contamination. The purge/sampling forms are enclosed in the Appendix.

Monitoring Well 20-V was installed as the Vertical Assessment Well, to a total depth of 115 feet. Cuttings were screened by OVA, and a sudden drop to 0 ppm was interpreted to indicate that the well was below the contaminated zone. A five-foot well screen was set to obtain a water sample from a depth of 110 feet to 115 feet. The analytical results showed that the groundwater is contaminated deeper than 115 feet below land surface, there. Minor amounts of VOA constituents confirm that the bottom of the plume is still below the well screen, due to the strong pumping effects of supply well W-1.

Table 5 - WELL CONSTRUCTION DATA

Teer Company, Durham, N.C.; Geonetics Corp. Project #10293

WELL#	CASING DIAM. (in.)	TOTAL DEPTH (ft.)	SCREEN DEPTH FROM TO	SLOT SIZE (in.)	TOP OF CASING ELEVATION (ft.)
W-1	6 **	260	18.0 - 255.0	*	323.66
W-2	6 **	400	24.0 - 395.0	*	327.52
W-3	6 **	160	23.0 - 160.0	*	323.3
W-4	6 **	162	40.0 - 110.0	*	324.81
W-5	6 **	200	22.0 - 160.0	*	316.48
MW-1	4	35	20.0 - 35.0	0.02	329.5
MW-2	6 **	52	14.0 - 52.0	*	334.04
MW-3	6 **	62	15.0 - 62.0	*	337.32
MW-4	6 **	55	40.0 - 55.0	*	323.38
MW-5	6 **	120	41.0 - 120.0	*	318.14
MW-6	6 **	67	26.0 - 65.0	*	315.09
MW-7	4	15	9.0 - 14.0	0.02	329.26
MW-8	4	28	18.0 - 23.0	0.02	315.62
MW-9	2	40	25 to 40	0.01	333.65
MW-11	2	50	35 to 50	0.01	327.87
MW-12	2	21	6 to 21	0.01	329.94
MW-13	2	65	50 to 65	0.01	326.48
MW-148	2	20	5 to 20	0.01	327.09
MW-14D	2	49	34 to 49	0.01	327.13
MW-15S	2	17	7 to 17	0.01	329.21

Table 5 - WELL CONSTRUCTION DATA

Teer Company, Durham, N.C.; Geonetics Corp. Project #10293

WELL #	CASING DIAM. (in.)	TOTAL DEPTH (ft.)	SCREEN DEPTH FROM TO	SLOT SIZE (in.)	TOP OF CASING ELEVATION (ft.)
MW-15D	2	40	25 to 40	0.01	329.53
MW-16S	2	13	3 to 13	0.01	333.91
MW-16D	2	61	46 to 61	0.01	330.8
MW-17S	2	12.5	2.5 to 12.5	0.01	327.59
MW-17D	2	22	7 to 22	0.01	327.3
MW-18S	2	13	3 to 10	0.01	328.43
MW-19	2	13	2 to 10	0.01	327.82
MW-20V *	6" / 2"	115	110 to 115	0.01	329.88
MW-20A	2	33	18 to 33	0.01	329.58
MW-21A	2	70	45 to 70	0.01	328.5

V \* = VERTICAL ASSESSMENT WELL; 6" OUTER CASING TO 100'; 2" CASING TO 110'; 2" SCREEN 5' LONG TO 115'

CLUSTER WELLS - "D" = DEEP WELL; "S" = SHALLOW WELL

A = ALTERNATE WELL

<sup>\* =</sup> SCREEN NOT INSTALLED, OPEN TO ROCK

<sup>\*\* = 6&</sup>quot; STEEL CASING

Southern Testing & Research Laboratories, Inc. performed the requisite analyses: EPA Methods 601, 602, 625 for organic hydrocarbons, and 239.2 for lead. PhosLab, Inc. performed Quality Control checks on the principal lab. Table 6 is a summary of the laboratory analyses for the groundwater samples. The Certificates of Analyses and Chains of Custody are contained in the Appendix.

Figure 7 delineates the configuration of the contamination plume using a benzene isopleth contour of 1 ug/L (ppb), the current N.C. regulatory limit. The plume is elongated in an easterly direction. Cross-sections A-A' and B-B', Figures 8 and 9, illustrate the groundwater contamination distribution beneath the site in the vertical dimension. It will be shown in the next section that the pumping influence of Supply Well 1 (W-1) has, for a considerable period of time, drawn the contamination downward and toward itself. Also, that this effect is beneficial to the remediation of the site.

The highest concentrations found were in MW-2. This is not a surprise in that it is situated closest to the tanks and the dispensers. The elevated concentrations behind and east of the former service station are most likely due to the influence of the regional eastward gradient and the surface water drainage ditch. Even though supply well (W-1) is 260 feet deep, it became contaminated because of the very short well casing and grout seal, to only 18 feet. Each time the pump would come on, it would draw shallower groundwater into the open bore. Otherwise, the contamination might have remained within 50 to 60 feet of the land surface. The volume of contaminated groundwater was estimated by surface area times average depth times a 40 percent porosity of the rock. The estimate is 4.16 million cubic feet (31.12 million gallons) of contaminated water.

It was noticed on some the analytical reports for EPA Method 601, that certain solvent constituents were detected. For example: trichloroethylene (TCE) was found at 2.1 ug/L in W-1, and also 2.1 ug/L in W-5, both below the maximum allowable concentration (MCL) of 2.8 ug/L. Tetrachloroethylene (formerly perchloroethylene; PCE) was identified in MW-6 at 2 ug/L, and W-5 at 99 ug/L. The N.C. MCL for PCE is 0.7 ug/L. Both of these wells are at the far eastern end of our study area, outside the petroleum-contaminated zone. We were advised that these cleaning solvents were formerly used by the construction company which used to operate there.

Table 6 - SUMMARY OF GROUNDWATER ANALYSES
NELLO L. TEER Co. Durham Quarry, N.C; Geonetics Project # 10293

PARAMETERS> (EPA METHOD)>		BENZENES (602)	TOTAL VOA (602)	MTBE (602)	EDB (601)	IPE (601) (ug/L)	TOTAL PAH (625) (ug/L)	LEAD (239.2) (ug/L)	LAB
(UNITS) MONITORING		(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	LAD
	SAMPLE								
WELL #	DATE								
W-1	5/7/93	16	22.4	N/A	N/A	N/A	N/A	N/A	So.Testing
	5/7/93	35.8	51.2	51.6	N/A	N/A	N/A	N/A	PhosLab
	5/20/93	34.4	53.4	11.8	BDL	N/A	<mcl< td=""><td>&lt;1.0</td><td>PhosLab</td></mcl<>	<1.0	PhosLab
	5/20/93	11.3	22.7	BDL	BDL	BDL	N/A	BDL	Patterson
	10/ <i>5</i> /93	11.8	11.8	5.2	ND	ND	ND	< 0.05	So.Testing
W-2	5/18/93	ND	ND	ND	ND	ND	ND	<.05	So.Testing
W-3	5/19/93	ND	0.9	ND	ND	ND	ND	· <.05	So.Testing
W-4	5/18/93	ND	ND	ND	ND	ND	ND	<.05	So.Testing
W-5	5/20/93	ND	0.7	ND	ND	ND	ND	<.02	So. Testing
	5/20/93	BDL	BDL	BDL	<1.0	N/A	<mcl< td=""><td>&lt;1.0</td><td>PhosLab</td></mcl<>	<1.0	PhosLab
	<i>5</i> /20/93	BDL	BDL	BDL	BDL	BDL	N/A	BDL	Patterson
MW-1	5/20/93	BDL	BDL	BDL	BDL	N/A	<mcl< td=""><td>&lt;1.0</td><td>PhosLab</td></mcl<>	<1.0	PhosLab
	5/20/93	BDL	BDL	BDL	BDL	BDL	N/A	BDL	Patterson
	5/20/93	ND	0.7	ND	ND	ND	ND	0.027	So.Testing
MW-2	<i>5/7/9</i> 3	<i>5</i> 75	3244	N/A	N/A	N/A	N/A	N/A	So.Testing
	<i>5l'71</i> 93	672	3110	23	N/A	N/A	N/A	N/A	PhosLab
	5/20/93	541	2006	105	BDL	N/A	<mcl< td=""><td>&lt;1.0</td><td>PhosLab</td></mcl<>	<1.0	PhosLab
	5/20/93	252	571	BDL	BDL	BDL	N/A	BDL	Patterson
	5/20/93	353	877	ND	ND	ND	ND	<.02	So.Testing
MW-3	5/21/93	ND	ND	ND	ND	ND	ND	0.056	So.Testing
MW-4	5/18/93	ND	0.7	ND	ND	ND	ND	<.05	So.Testing

Table 6 - SUMMARY OF GROUNDWATER ANALYSES (CONT.)
NELLO L. TEER Co. Durham Quarry, N.C; Geonetics Project # 10293

PARAMETERS> (EPA METHOD)> (UNITS)>		BENZENES (602) (ug/L)	TOTAL VOA (602) (ug/L)	MTBE (602) (ug/L)	EDB (601) (ug/L)	IPE (601) (ug/L)	TOTAL PAH (625) (ug/L)	LEAD (239.2) (ug/L)	LAB
MONITORING	SAMPLE	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	LAD
WELL #	DATE_								
WELL#	DATE							<del></del>	
MW-5	5/7/93	ND	, ND	N/A	N/A	N/A	N/A	N/A	So.Testing
	<i>5</i> /7/93	BDL	BDL	BDL	N/A	N/A	N/A	N/A	PhosLab
	5/20/93	BDL	BDL	BDL	BDL	N/A	<mcl< td=""><td>&lt;1.0</td><td>PhosLab</td></mcl<>	<1.0	PhosLab
	5/20/93	1.5	12.4	BDL	BDL	BDL	N/A	BDL	Patterson
	5/20/93	ND	ND	ND	ND	ND	ND	0.069	So.Testing
MW-6	5/21/93	ND	ND	ND	ND	ND	ND	0.03	So.Testing
MW-7	5/21/93	ND	ND	ND	ND	ND	ND	<.02	So.Testing
MW-8	5/19/93	ND	ND	ND	ND	ND	ND	<.05	So.Testing
MW-9	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-11	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-12S	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-13	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-14S	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-14D	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-158	9/9/93	<b>10.7</b>	95.9	8.3	ND	ND	13	<0.05	So.Testing
MW-15D	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-16S	9/9/93	ND	ND	ND	ND	ND	N/A	N/A	So.Testing
MW-16D	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-17S	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-17D	9/9/93	ND	ND	ND	ND	ND	ND	<0.05	So.Testing
MW-18S	9/9/93	ND	1.3	ND	ND	ND	ND	<0.05	So.Testing

Table 6 - SUMMARY OF GROUNDWATER ANALYSES (CONT.)
NELLO L. TEER Co. Durham Quarry, N.C; Geonetics Project # 10293

			TOTAL				TOTAL		
PARAMETER	RS>	BENZENES	VOA	MTBE	EDB	IPE	PAH	LEAD	
(EPA METHO	)D)>	(602)	(602)	(602)	(601)	(601)	(625)	(239.2)	
(UNITS):	>	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	(ug/L)	LAB
MONITORING	SAMPLE								
WELL #	<b>DATE</b>	<del></del>							
MW-19	9/9/93	ND	ND	ND	ND	ND	ND	< 0.05	So.Testing
MW-20V	9/15/93	15	15	6.2	ND	14.2	ND	< 0.05	So.Testing
MW-20A	9/9/93	ND	1.8	7.3	ND	ND	ND	< 0.05	So.Testing
MW-21A	9/9/93	ND	ND	ND	ND	ND	ND	< 0.05	So.Testing

NOTES:

ND = NOT DETECTED

BDL = BELOW DETECTION LIMITS

ug/L = micrograms per Liter (ppb)

<MCL = LESS THAN MAXIMUM CONTAMINATION LEVELS</pre>

PhosLab, Inc. , LAKELAND, FL.

Southern Testing & Research Laboratories, Inc., WILSON, N.C.

Patterson Exploration Services, Inc., SANFORD, N.C.

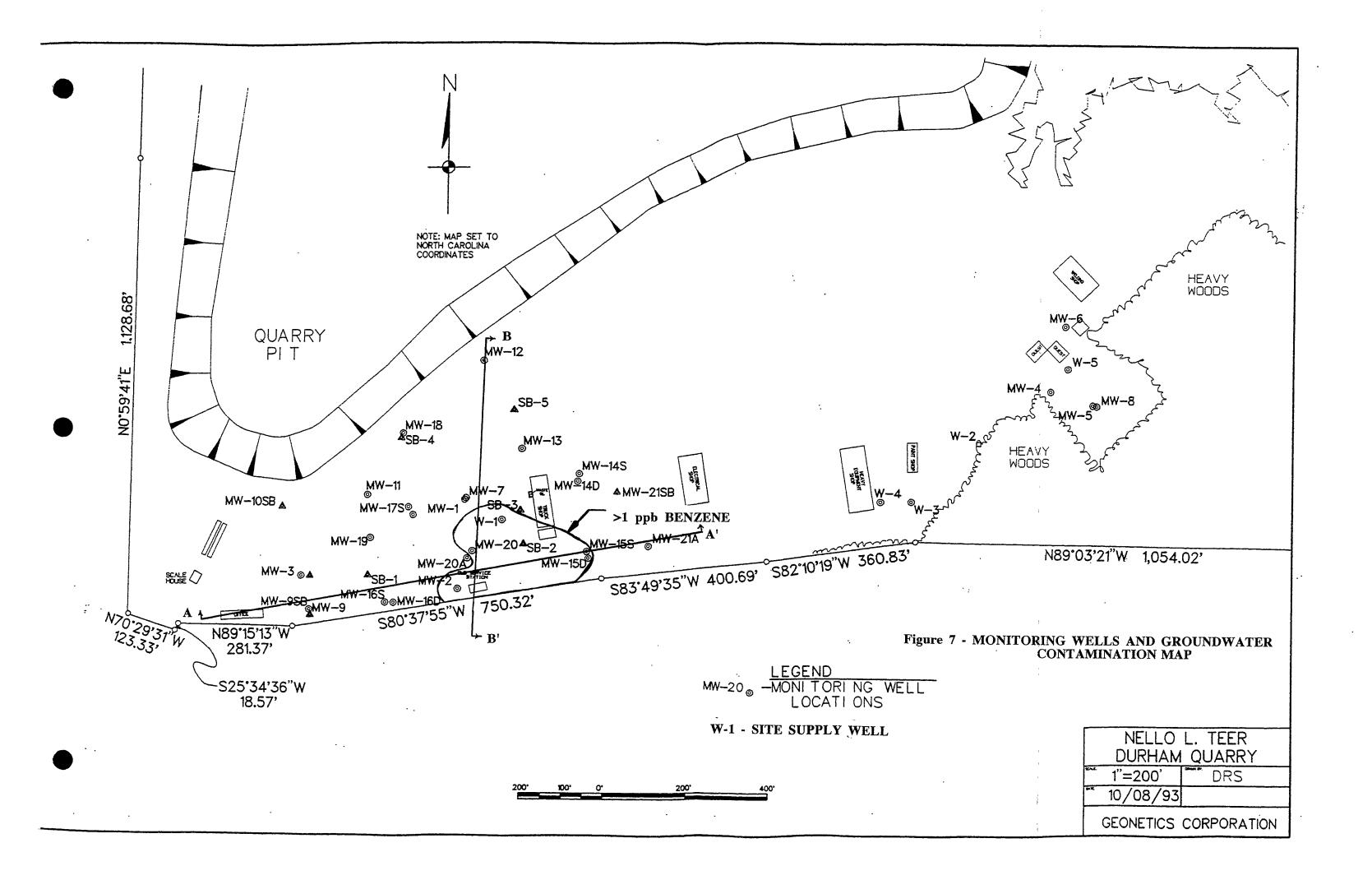
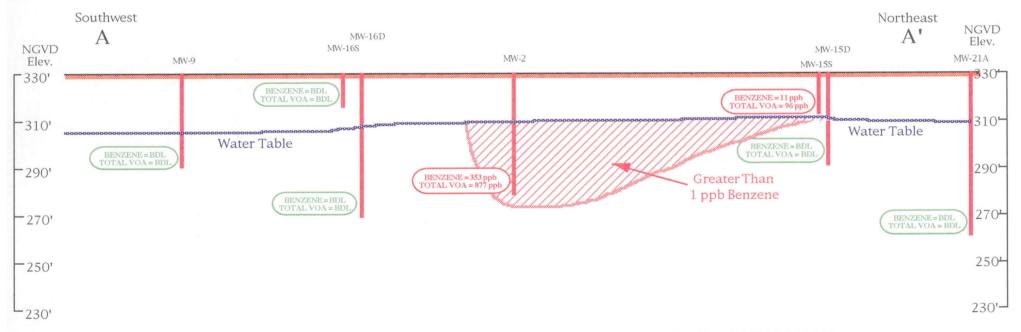


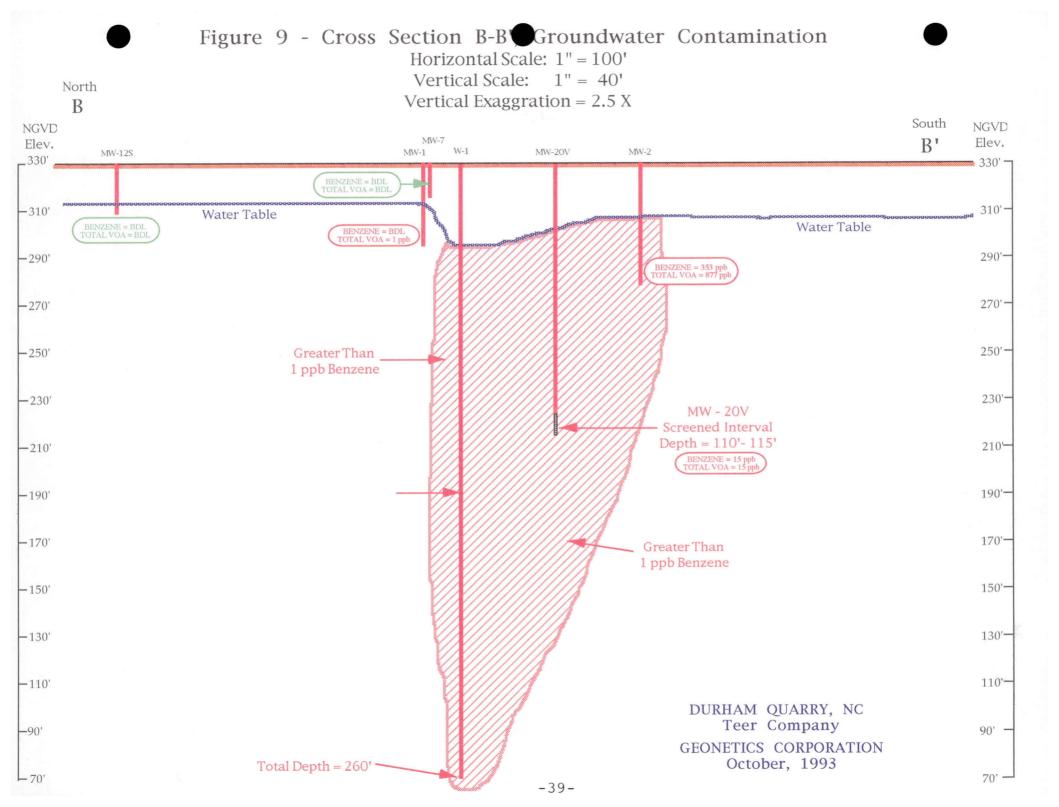
Figure 8 - Cross Section A-A', Groundwater Contamination

Horizontal Scale: 1" = 100' Vertical Scale: 1" = 40' Vertical Exaggeration = 2.5 X



DURHAM QUARRY, NC Teer Company

GEONETICS CORPORATION October, 1993



## Groundwater Levels & Flow Directions

North Carolina Registered Land Surveyor, Doug Helms of Winston-Salem, was subcontracted to survey the monitoring well locations, and measuring point elevations. The site maps and property boundaries have all been redrawn to comply with the North Carolina Coordinate System.

Not only did the installation of these new wells allow for the water quality sampling, but also for determination of the groundwater flow directions. Water levels were measured on five occasions from April to October, 1993. These data are listed in Table 7. The Water Level Data Forms are contained in the Appendix. Two unconfined (water table) aquifers were encountered in this investigation. The shallow water levels, less than 20 feet, represent a perched aquifer system; whereas the deeper wells are finished into the principal water table aquifer.

Figures 10 and 11 are the Water Table Contour Maps. They illustrate that the groundwater flows in different directions at different locations across the study area. Again, note the strong drawdown influence of supply well W-1, in the vicinity of the former service station site. This pumping effect has created a cone of depression and pulled water from all directions around it, thus reducing the spread of contamination.

Gradients range from 0.014 (ft/ft) at the eastern end, to 0.214 (ft/ft) in the area around W-1. Two slug tests were performed to estimate hydraulic conductivity. The slug test in MW-1 gave a result of 8.5 X 10-5 cm/sec. MW-6 test resulted in an hydraulic conductivity of 5.5 X 10-3 cm/sec. The slug test values are in close agreement for similar sedimentary lithologies as reported by traditional authors (Davis and DeWiest, 1966) (Chow, 1964).

As this report was being finalized, a recovery and drawdown test was in progress to study the effects of pumping W-1 upon MW-20V. The base of the contaminated plume may rise above the screened interval in MW-20V and yield clean water. These hydrologic results will be helpful in the remediation design and will be reported in the CAP.

Table 7 - WATER LEVEL DATA

Teer Company, Durham, N.C.; Geonetics Corp. Project # 10293

		DATE							
MONITOR	MEASURE PT.	WATER TABLE ELEVATION							
WELL #	ELEVATION	4/17/93	4/24/93	4/30/93	5/15/93	10/11/93			
W-1	323.66	305.17	307.02	306.34	306.81	288.18			
W-2	327.52	287.81	N/A	292.65	293.39	275.17			
W-3	323.30	N/A	N/A	N/A	N/A	286.11			
W-4	324.81	288.35	288.75	292.08	292.32	286.00			
W-5	316.48	288.24	287.82	290.76	290.99	284.81			
MW-1	329.50	315.68	315.76	315.25	315.23	311.71			
MW-2	334.04	311.63	311.68	311.68	311.40	305.93			
MW-3	337.32	311.99	312.14	311.73	311.20	305.55			
MW-4	323.38	287.69	287.54	292.36	292.86	286.53			
MW-5	318.14	293.45	292.83	294.23	294.24	287.28			
MW-6	315.09	286.36	286.64	291.21	291.32	285.01			
# MW-7	329.26	319.14	319.23	318.92	318.92	317.45			
MW-8	315.62	292.40	292.14	291.01	292.30	287.32			
MW-9	333.65	*	*	*	*	305.08			
MW-11	327.87	*	*	*	*	290.73			
# MW-12	329.94	*	*	*	*	320.51			
MW-13	326.48	*	*	*	*	299.52			
# MW-14S	327.09	*	*	*	*	310.75			
MW-14D	327.13	*	*	*	*	309.70			
# MW-15S	329.21	*	*	*	*	318.93			
MW-15D	329.53	*	*	*	*	310.67			
# MW-16S	333.91	*	*	*	*	DRY			
MW-16D	330.80	*	*	*	*	302.49			
# MW-17S	327.59	*	*	*	*	323.91			
MW-17D	327.30	*	*	*	*	324.03			
# MW-18	328.43	*	*	*	*	323.70			
# MW-19	327.82	*	*	*	*	325.73			
MW-20A	329.88	*	*	*	*	305.97			
MW-20V	329.58	*	*	*	*	<b>2</b> 99.46			
# MW-21A	328.50	*	*	*	*	309.02			

#### NOTES

Water Level Measurements taken with Electric Tape

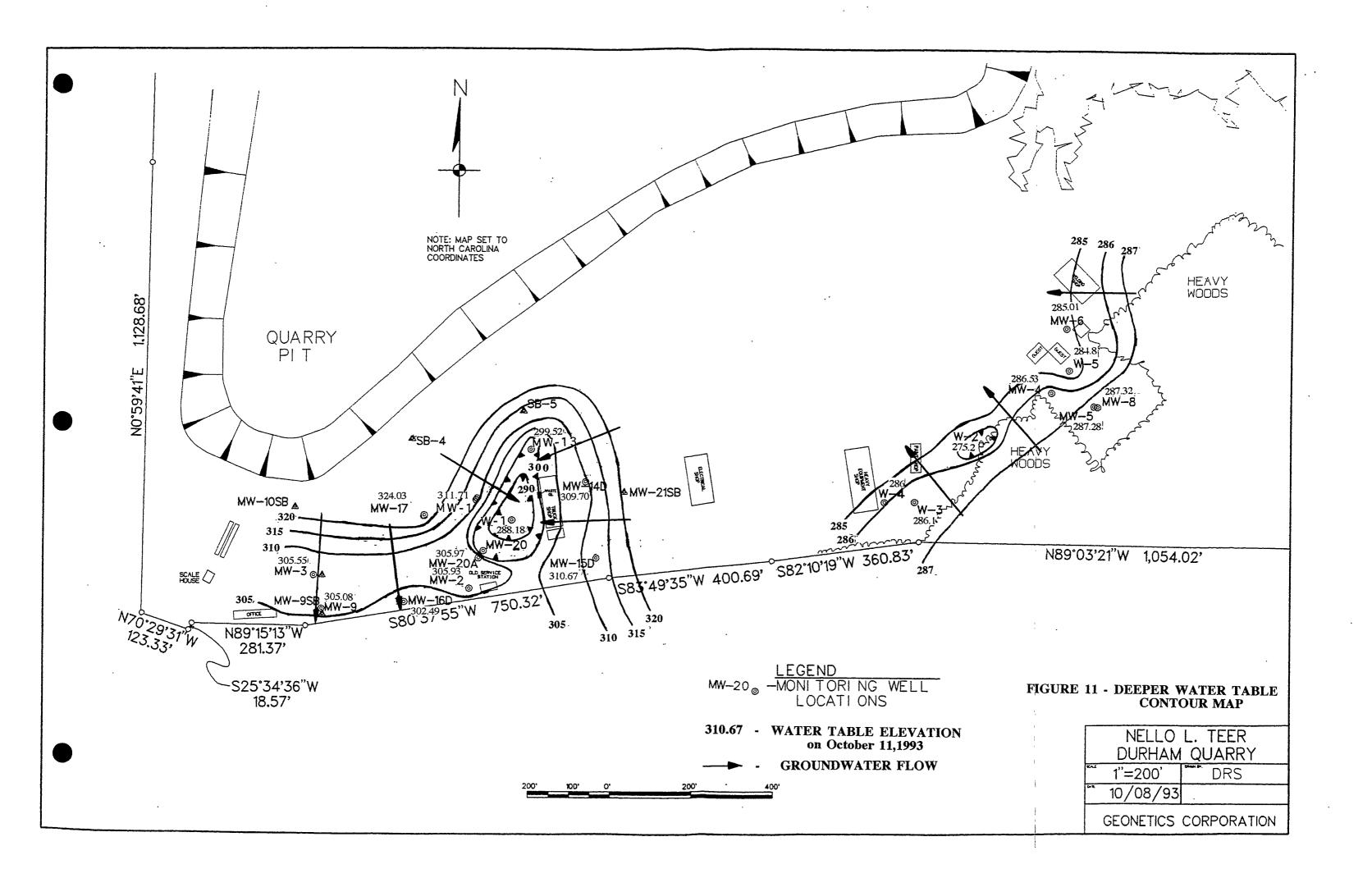
N/A = Water Level not measured on this date

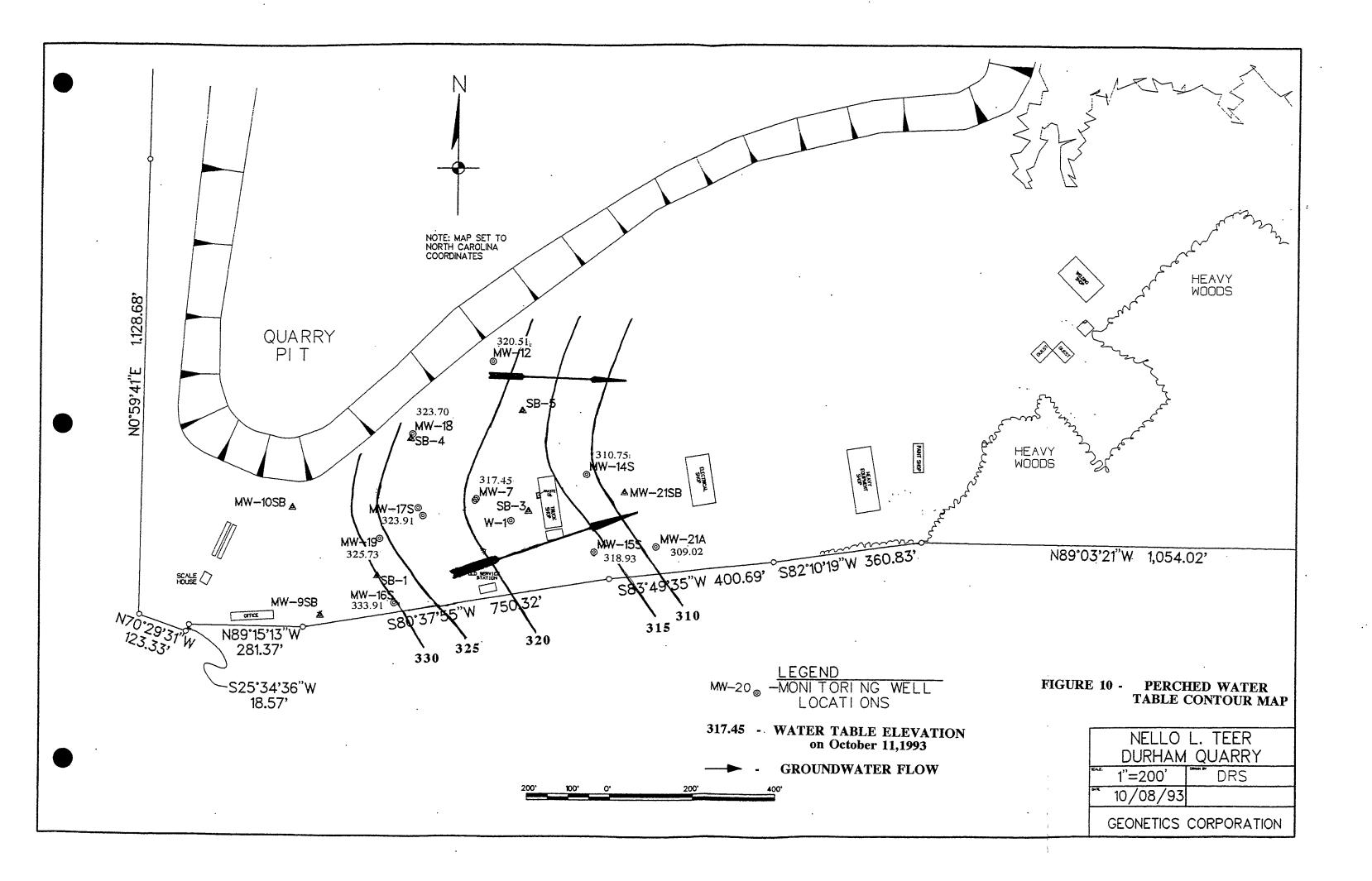
<sup>\* =</sup> Wells not Installed at this time

<sup># =</sup> Shallow Wells used on Perched Water Table Contour Map

D = Deeper Well

V = Vertical Assessment Well





### CONCLUSIONS AND RECOMMENDATIONS

The category of contamination has been shown to be primarily from gasoline, diesel and oil leaks and/or spillage. No free product was found upon the groundwater from these tank, line, or dispenser leaks. Residual soil contamination is documented to be outside of the area of the tanks, and has migrated toward supply well W-1. It appears that the fuel travelled downward through the silt-clay soils until intersecting the water table.

The groundwater flow directions have been confirmed to be in various directions. Hydraulic conductivity is very low, estimated to be between 10-3 and 10-5 cm/sec. The contaminated groundwater has migrated northward toward W-1, and eastward following the shallow water gradient. The contamination appears to have remained on site.

Following, is the Preliminary Corrective Action Plan (CAP) for this site. Corrective action will be addressed for both soils and groundwater:

Soils: The estimated quantity of contaminated soils is 137,778 tons (91,852 cubic yards). The contaminated soils are located around the former service station, the former asphalt plant, the existing septic tank drain field, in front of the truck shop, and along the surface water darainage ditch. The cost to remediate the soil through on-site bioremediation is estimated to be about \$190,000.

Groundwater: The estimated quantity of contaminated groundwater is about 31.12 million gallons (4.16 million cubic feet). The groundwater remediation equipment will be sized to process as much water as can be handled by the recovery pump in well W-1. It is hoped that the system will be able to operate at least 10 to 20 gpm in order to expedite cleanup of this site.

The contaminated groundwater can be remediated using an air sparging system and/or granular activated charcoal. The system utilizes off-the-shelf technology and equipment to remove pollutants from the contaminated aquifer. The contamination will be removed from the groundwater to an acceptable level as opposed to complete removal. This is to be accomplished by passing the groundwater through an air sparger (low profile air stripper) and then through a two-stage carbon filter giving a removal efficiency that should reduce all contaminants in the water to a level at or below regulatory limits. Treated groundwater will be either recharged back into the surficial aquifer or permits for surface discharge modified.

Currently, one recovery well is planned as the existing well located within the contamination plume. Additional recovery wells can be installed should the planned wells fail to give ether the needed radius of influence or the gallons per minute (gpm) needed to expedite the cleanup in a timely manner. The cleaned groundwater will be recharged back into the surficial aquifer through an infiltration gallery or injection wells or discharged into surface systems.

This site is eligible for reimbursement from the Commercial Trust Fund. To remain eligible, the Corrective Action must proceed as directed by the Raleigh Regional Office and to mitigate the spread of contamination, in accordance with 15A NCAC 2N. Geonetics Corporation is now preparing the Corrective Action Plan. The Corrective Action will commence as soon as this CAP is approved. If there should be any questions or need for clarifications, please contact one of our offices listed on the cover page.

### REPORT CERTIFICATION

The work activities relating to this Comprehensive Site Assessment at the Durham Quarry were performed under the supervision of three licensed Professional Geologists: Dr. Arthur W. Hayes, Geonetics' President and Senior Hydrogeologist, and Donald R. Smith, Division Manager, based in Lexington, N.C., and Steven S. Edgerton, Chief Geologist, The Teer Company. The information contained herein, and the interpretations derived, follow accepted and approved professional practice, and are true and correct to the best of our knowledge. It is understood that interpretations and conclusions are derived from dated samples and measurements, and that conditions may change through time and in three dimensions within the Earth.

**GEONETICS CORPORATION** 

Donald R. Smith, P.G.

Date: 29 dei 93

Arthur W. Hayes, Ph.D., P.G.

Date: 1) 12 8, 1993

#### REFERENCES

- BURT, E.R., Carpenter, P.A., III, McDaniel, R.D., and Wilson, W.F. (1978): Diabase Dikes of the Eastern Piedmont of North Carolina; N.C. Geological Survey, Information Circular 23, 12 p. and map.
- CHOW, Ven Te (Editor)(1964): Hankbook of Applied Hydrology; McGraw-Hill Book Company, New York.
- DAVIS, S.N. and DeWiest, R.J.M. (1966): Hydrogeology; John Wiley & Sons, Inc., New York, 463 p.
- HOFFMAN, C.W. and Gallagher, P.E. (1989): Geology of the Southeast and Southwest Durham 7.5 minute Quadrangles, N.C.; N.C. Geological Survey, Bulletin 92, 34 p.
- KRYNINE, D.P. and Judd, W.R. (1957): Principles of Engineering Geology and Geotechnics; McGraw-Hill Book Company, New York, 730 p.
- N.C. DIVISION OF ENVIRONMENTAL MANAGEMENT (1993): Groundwater Section Guidelines for the Investigation and Remediation of Soils and Groundwater; internal publication, March 10, 1993, 95 p.
- PARKER, John M., III (1979): Geology and Mineral Resources of Wake County; N.C. Geological Survey, Bulletin 86, 122 p.
- WILSON, W.F. and Carpenter, P.A., III (1981, revised): Region J Geology A Guide for North Carolina Mineral Resource Development and Land Use Planning; N.C. Geological Survey Reg. Geol. Series 1 (orig. 1975), 45 p.

# **APPENDIX**

Soil Boring Logs
OVA Measurements
Laboratory Soil Analyses
MW Construction & Drilling Logs
Monitoring Well Data Forms
Groundwater Analyses
Water Level Measurements

# APPENDIX to the COMPREHENSIVE SITE ASSESSMENT

ON SHEEF IN

at

DURHAM QUARRY
Denfield Street
Durham County
Durham, North Carolina
DEM Facility # 0-012984
Groundwater Incident # 9357

for
Teer Company
Post Office Box 13983
Research Triangle Park,
North Carolina 27709-3983

by

GEONETICS CORPORATION

Payne Road Route 10, Box 2620 Lexington, N.C. 27292 Telephone (919) 764-9225 Suite 1 5120 S. Lakeland Drive Lakeland, Florida 33813 Telephone (813) 646-2644

Geonetics Corporation

October 28, 1993